GROUTING AND GROUTING FORMULATIONS

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A Not-for-Profit Knowledge Centre
Grouting in concrete and masonry structures is one of the most critical applications in the construction chemical division that any engineer may face. The required hardened properties of the grout materials depend on fluid properties, set properties at the wet stage of the material and mechanical properties of the grout mix at the hardened stage. When placing the grout material into a void or a crack or inside any cavity depends upon the flowability of the material and required pressure that may fill every nook and corner of the space. Without knowing the internal grouting space such as the position and space distribution inside the structure; grouting is often made with some assumptions or with the practical experiences of the skilled applicator. In some cases, grouting may fail if all the void spaces are not filled in.

There are mainly three different types of grouts: cementitious-based structural grouts, epoxy resin-based grouts and polyurethane-based grouts, apart from many specially formulated chemical grouts for specific applications for concrete and masonry structures. The success of cementitious grouts depends upon the selection of the grout mix, admixture such as expanding agents, accelerators, retarders, thixotropic agents, air entrainers and plasticizers as well as hole spacing, mixing and placing of the grout. When the filler is added to the grout, high speed mixing using a shearing action is necessary to ensure thorough distribution and wetting of cementitious particles throughout the mix. A grout having colloidal characteristics is thus formed, and the segregation of the sand is prevented which helps to reduce the bleeding of the mix. When using a two component epoxy or polyurethane based grouts, a thorough mixing of both resin and hardener compounds is required. While mixing such type of grouts, care should be taken to as much quantity that can be used within its pot life, or else it will thicken very quickly.

Pressure grouting is used in new construction as well as in the repair and rehabilitation of all types of existing structures. In structural work, there are various types of grouting for which each contractor or applicator does specific work because of the complex nature of each job. In new construction, grouting is part of virtually all post-tensioned concrete work, wherein the spaces remaining in the tendon ducts after tensioning are filled with a cementitious grout. Grouting of tall vertical prestressed ducts prevents corrosion of prestressing tendons and provides an efficient bond between the tendons and concrete members. Construction joints of all new massive concrete structures are frequently filled with a cementitious grout once the initial major shrinkage has occurred. Void spaces under and around precast elements are also filled with cementitious grouts in order to attain monolithic construction. The bearing spaces below base plates of all types and sizes are supported by injected grout. The bases of wide variety of tanks, machinery and mechanical equipment are injected with cementitious injection grouts or free flow grouts. Resinous grouts such as epoxy are injected into narrow joints to bond different elements of concrete to arrest fine cracks inside the concrete. Both cementious and resinous grouts are widely used in the repair and rehabilitation of concrete and masonry structures. New reinforcement bars are sometimes placed in holes drilled into masonry/concrete to grout with polyester resin injection to provide additional tensile capacity to the section and similarly to concrete structures as anchor bars. Epoxy injection into cracks represents one of the largest areas of structural grouting. Masonry and RCC structures that are damaged due to earthquakes also need epoxy injections with some flexibility. The control of water leakage into structures and form within water-retaining structures is possible by injection with polyurethane grouts. In new construction of water retaining structures, waterbar is preplaced at all joints prior to concrete placement. Once the concrete is cured the section is injected with chemical solution grout. The most commonly used are based on urethane resin, which can be formulated into a solid, flexible gel or either rigid or flexible foam. Several urethanes are water activated and will react soon as they come in contact with water. Although urethane-based grouts are the most commonly used, acrylic resins are also used, especially for injection into low-permeability, porous area and very fine crack networks. Silicate injection grouts are used for preventing rising dampness in masonry structures. Cementitious suspensions are injected into masonry to have the integrity of the structure where as cementitious slurries are injected into masonry structures to fill small to large voids.

One has to understand various tests that need to be carried at laboratory such as flowability, bleeding, setting time and volume change, drying shrinkage to satisfy the requirements of the Standard. The setting of grouts may be based on many different chemical reactions involving different binders such as Portland and non-Portland cements, sodium silicate, sodium aluminates, polyacrylamides, polyacrylates, polyurethanes and a wide range of resins. Similarly, volume change is a critical parameter especially in chemical grouts. It is not possible to consider all the volume change mechanisms that may occur in chemical grouts. But more important is grout specifications which must be appropriate to the grout material for particular usage under various conditions. This can be achieved with the right kind of formulations of grout material in terms of desired properties and long term durability. The laboratory test is only for conformity of the material, however, the performance of material may vary at the actual site which depends upon many factors. In fact, it is very difficult to simulate the exact condition in the laboratory during the trial with actual site conditions.

Earlier, we had also published a special issue on grouting in our ReBuild describing the various test properties and application methods. But in this issue of ReBuild, we are focusing more on the formulation and specifications and hope our readers will find it extremely useful.
Comparison of Chemical Grout Properties: Which grout can be used where and why?


1.0 Introduction
Grout is a fluid material which is designed to be introduced into a cavity for the purpose of filling it and which will subsequently harden to give specific physical properties (Fig.1). Grout is generally a mixture of water, cement, sand, often colour tint, and sometimes fine gravel (if it is being used to fill large spaces such as the cores of concrete blocks). Unlike other structural pastes such as plaster or joint compound, correctly-mixed and -applied grout forms a waterproof seal. Although both are applied as a thick emulsion and harden over time, grout is distinguished from its close relative mortar by its viscosity; grout is thin so it flows readily into gaps, while mortar is thick enough to support not only its own weight, but also that of masonry placed on top of it. Grout varieties include tiling grout, flooring grout, resin grout, non-shrink grout, structural grout and thixotropic grout. Tiling grout is often used to fill the spaces between tiles or mosaics, and to secure tile to its base. Tiling grout is also cement-based, and comes in sanded as well as unsanded varieties. The sanded variety contains finely ground silica sand; unsanded is finer and produces a non-gritty final surface. They are often enhanced with polymers and/or latex. Structural grout is often used in reinforced masonry to fill voids in masonry housing reinforcing steel, securing the steel in place and bonding it to the masonry. Non-shrink grout is used beneath metal bearing plates to ensure a consistent bearing surface between the plate and its substrate. Grout is also used in construction to embed rebars in masonry walls, connect sections of pre-cast concrete, and fill voids.

Fig. 1: View of flow of a grout material

The various requirements of grouts need to be considered as follows:
• The nature of the void/void system to be grouted.
• The intrinsic properties of the grout to be used including its health and safety requirements.

The on-site preparation and QC procedures.
• The fluid properties of the grout including its behaviour during and after injection.
• The properties of the hardened grout.
• The test methods for the hardened grout.
• The durability of grout.

2.0 Material Properties
The basic properties of a grout material are dimensional stability, strength, elastic modulus and thermal properties.
• Dimensional stability: It refers the property of the material to change in shape or volume.
• Strength: The strength of the material refers to the magnitude of a stress, or a load, the material can withstand without rupture. In structural application both compressive and tensile/bond strength are more important. However the strength required to match the required strength or original strength of the structures.
• Elastic modulus: The modulus of elasticity refers to the stiffness of the material. Wherever the high strength is required, the elastic modulus of the material should be higher.
• Thermal properties: The effect of temperature on resinous grouts varies greatly, therefore must be considered. In extreme temperature variations, special formulations or amending admixtures should be used to negate the effect of temperature on grout materials.

3.0 Grout Rheology
The term rheology is used to describe all the properties of a particular grout, both as initially mixed and in the hardened state.

3.1 Consistency
The consistency is known as ability of grout to flow. Grout can range in consistency from a near-water or very-thin-paint consistency to an almost thick stiff mortar or thixotropic consistency, depending on the application and desired workability.

3.2 Material Behaviour
• Viscosity: When the grout is in the form of fluid, its viscosity is the important property. When in solid form, it is characterized by the elastic modulus which denotes stiffness of the substance. When the grouts are in the intermediate group of fluid and solid, its property is described by viscoelastic.
• Thixotropy: Many grouts behave thixotropic fluids, which is the behavior of the fluid as an immobile paste or gel when at rest, but as fluid upon application of sufficient energy to start it moving.
• Flowability: The grout materials flow freely at lower sustained pressure. Temperature has significant influence on the viscosity of the material.
Mobility: Mobility is the most important property that denotes the property of material to travel through the delivery system and into desired voids. Even some thick and no slump grout can be of higher mobility. The slump test is most commonly used to check workability and mobility of the grout.

Penetrability: Penetrability of the grout depends on viscosity and wetability of the material. The grain size of the grout affects its ability to penetrate.

3.3 Rheological properties
- Cohesion: A low cohesion of the material will help for high penetration. When travel of the material need to be restricted the cohesion of the material should increase.
- Bleeding: The grouts tend to settle out of the solution leaving excess mix water on the top of the settle solids which is known as bleed. This can be reduced by good dispersion of the solids through high shear mixing. To prevent bleeding prior to injection, the grout is agitated continuously after mixing.
- Temperature: Higher the temperature, lower the viscosity and vice-versa and more significant in case of polymer resins. Viscosity of the material decreases as pressure increases, but not significantly as compared to temperature. It also affects setting time, as well as the developed strength and long-term durability.
- Setting time: The control of time is required for setting of the grout. Where many holes are to be grouted, the material must set or immobile before adjacent holes are drilled. The admixtures can be added to accelerate or delay the setting time of grout. In most of the cases the grouts are two-components, with the set or hardening time being a function of the proportion of the reactant; i.e. hardener to base.
- Solubility: The rapid dilution of grout materials is essential which decreases the mixing time.

4.0 Grouting in concrete structures
While doing the grouting in concrete structures it is important to consider various parameters such as the purpose of the grouting, understand the defects, width and depth of crack, required strength and other properties of grout material, method of grouting, grouting equipment etc. The different purposes of grouting are as follows:
- Filling structural cavities and voids
- Filling cracks and joints
- Grouting bed plates
- Contact grouting
- Post tension tendon grouting

Due to bad workmanship or improper compaction during the casting of the concrete, the structure may develop honeycombs or cavities. Cementitious grouts are preferable for virtually all voids filling in concrete, as they set up and harden to similar physical properties as host material.

Grouting methods are widely used to fill cracks and joints both in repair of existing structures and in new constructions. The purpose of such work is to block water flow, to provide a structural filler to resist compressive forces, or to bond the two sides of a fault together to create a monolithic section. Portland cement is the most common cementing agent in grout, but urethane- and epoxy-based formulas are also popular depending on location and application of the structure.

4.1 Cementitious Grout
Portland cement-based grouts come in different varieties depending on the particle size of the ground clinker used to make the cement, with a standard size of around 15 microns, microfine at around 6-10 microns, and ultrafine below 5 microns. Finer particle sizes let the grout penetrate more deeply into a fissure. Because these grouts depend on the presence of sand for their basic strength, they are often somewhat gritty when finally cured and hardened.

Ultrafine cementitious grout, also called microfine grout, has been produced for almost 30 years and has been refined over that time span to repair dams, tunnels, and bridge supports. Ultrafine cementitious grouts are composed of a finely ground mixture of Portland cement, pumice, and dispersant. A key characteristic of ultrafine grout is the particle size (typically in microns).

Most cementitious compositions shrink and thus do not develop very high bond strength. So they are employed primarily for filling applications. For opening of less than 3 mm wide, neat cement grouts are used. As the gap widens, sand can be added in the mix.

The particle size is critical in determining the permeability of the ultrafine grout into the structure. Latex type bonding material is added to improve the bond strength. This latex material can act as bond breaker once it has skinned over, which occurs quite rapidly. It should be thus added at last during the mix of ingredients and injected promptly. Usually the open time varies within a range of 10 to 30 minutes but also depend on temperature which affects the setting of the grout. Fly ash and natural pozzolanic materials are added in structural grouts which help in improving the pumpability and penetrability and contributes to improve the final compressive strength. Viscosity modified admixtures are also added in the structural grout to provide stability. Properties essential in cementitious grouts are as follows:
- Non-shrink
- High flow
- High strength (early and ultimate)
- Corrosion resistance
- Resistance to high dynamic load
4.2 Epoxy Grouts

Epoxies consist of two components that react with each other forming a hard, inert material. Their bond with most substances is great enough to overcome their very different hardness and modulus. Part A consists of an epoxy resin and Part B is the curing agent, sometimes called the hardener. The curing agent selection plays the major role in determining many of the properties of the final cured epoxy. These properties include pot life, cure or drying time, penetration and wet-ability. Curing agents come in many different chemical flavors, all based on amines or amides. The well known adhesive strength of epoxies is due to strong polar bonds it forms with the surfaces it comes in contact with. On dry surfaces the bond between the surface and the epoxy displaces the air, which is fluid. The same can be true on wet surfaces and even completely underwater. As with all adhesive applications, the cleanliness of the surfaces or cracks to is often the paramount limitation. Underwater applications are becoming more common with the advancement of these types of products.

4.2.1 Required properties of epoxy grout

Viscosity and thixotropy - Low viscosity is required for epoxy to penetrate cracks without using high injection pressure. Typical viscosities for liquid epoxy injection adhesives range from 100 to 500 cps at 25°C. However, if injection adhesives with viscosities lower than 100 cps are used, the adhesive can penetrate into the concrete so far that it leaves a starved bond line. In this case, there must be a continual reservoir of adhesive available to the crack until the adhesive gels fill to the bond line. Liquid adhesives without thixotropic properties will also drain out of a crack, even into sub grades, if all faces of the crack are not sealed prior to filling the crack. For cracks where all faces cannot be sealed; a thixotropic or pseudoplastic adhesive should be used which will stay in the crack without constraint.

Concrete temperature - Cracks in concrete open and close as the temperature of the concrete changes. If a crack cannot be injected while it is in its widest position, an injection adhesive should be chosen that cures fast enough to resist the tensile forces that result when the crack widens from temperature change.

Flexibility - The use of a low modulus flexible adhesive in a crack will not allow any significant movement of the concrete structure. The effective modulus of elasticity of a flexible adhesive in a crack is substantially the same as that of a rigid adhesive.

Creep Resistance - Frequently the adhesive in a bonded crack will be subjected to sustained loads. These loads may be external or they may be caused by restraints on a structure that is undergoing cyclic temperature changes. Unless it can be determined that the adhesive in a crack will not be subject to sustained loads, an adhesive conforming to ASTM, C 881 Type IV should be used.

4.2.2 Application

One of the main uses is to repair concrete. These two-component epoxy resins have better expansive properties than some hydrophobic type products. One unique characteristic is that these types of products do not require water for the reaction to begin; this reaction takes place when Part A comes into contact with Part B in the delivery system. The material is non-toxic (0% VOC's).

4.2.3 Limitations

Only mix the amount of epoxy that you can use in ½ the pot life. Materials will start to thicken at this point and are more difficult to work with. Keep in mind that large batches will set up faster than smaller batches. Start by mixing small batches and then increase your batch size slowly, to insure that you do not loosen your mix. It is very important to mix all epoxy thoroughly as is called for in each product's data sheet. When mixing epoxy resins, always mix Part B into Part A, scraping all of the resin out of both containers. Injection epoxies can be mixed with a two component in-line mixer. Do not stir epoxy by hand only; instead use a low speed drill mixer of proper size. Typically warmer epoxies set much faster than cooler epoxies. Generally epoxy resins will have either good chemical resistance or good heat resistance, but not both. Another characteristic of this type of product in its cured state is the lack of flexibility, and the system might be prone to failure if movement occurs due to seismic activity, and or expansion/contraction.

4.3 Polyurethane Chemical Grouts

Polyurethane chemical resins used for grouting started with only two water activated materials which were and still are used for a wide variety of applications, mainly for sealing active water leaks. These early systems were the basis for what has now evolved into a wide variety of resins, which are available at numerous manufacturers. These first two resins represented both of the systems that are currently available (they are called hydrophobic and hydrophilic) resins while the number of resins with differing properties has grown tremendously. Those products are chemically reacted urethanes and don’t use water at all. They can be used as joint fillers which are basically dry at the time of application.

4.3.1 Hydrophilic Grout Systems

Hydrophilic expansive grouts react upon contact with water, absorb water while curing, and cure to a flexible foam or gel. They are generally used to seal leaks in joints or cracks and to repair leaking water-stops. Hydrophilic expansive foam grouts chase and absorb the water in the crack and in all of the fractures that branch off from the main crack. A key characteristic of any liquid is its viscosity (cps) compared to water. Water has a cps of 1,
where hydrophilic expansive grouts could range from 300-2500 cps. The lower the cps (the lower the viscosity) of any hydrophilic expansive grout, the better suited it is for tighter cracks (for better penetration) and for applications that might require greater travel. The higher the cps (the higher the viscosity) of any hydrophilic expansive grout, the better suited it is for high flow/high volume applications so as not to become diluted.

4.3.1.1 Applications

Hydrophilic expansive foam grouts have an initial cure and final cure. The initial cure is the time it takes for the polyurethane grout to foam up, and the final cure is the time it takes for the grout to fully expand. This final cure time, which may take up to 12 hours, is critical to the success of the grouting process. Hydrophilic foams have been successfully used in above-grade and below-grade applications, but hydrophilic gels should be used below grade as they will shrink in a dry environment.

Hydrophilic expansive gel grouts can be mixed with large amounts of water to offer an alternative grouting material in areas such as curtain grouting and manhole grouting. Most polyurethane grouts are considered to be “non toxic” although safe handling procedures should be closely followed with these and all other chemicals.

Hydrophilic expansive foam grouts are typically single component products requiring small delivery systems for the injection process. These types of grouts are used in below grade structures, basements, and other areas that are often wet, such as subways and interior portions of a concrete dam. If injecting a hydrophilic gel grout for manhole grouting, or curtain grouting a multi-ratio delivery system would be needed. Pumping systems for hydrophilic foam grouts tend to be high pressure and low volume, while the gels utilize high volume and lower pressure systems.

Some hydrophilic foam grouts are certified to be used with potable drinking water systems.

The expansion rate of hydrophilic foam grouts can be up to 5 to 7 times its original volume and hydrophilic gels typically do not gain volume upon curing.

4.3.1.2 Limitations

Hydrophilic expansive foam grouts will stick to concrete. They will stretch in a moving crack and are generally used for crack sealing or filling voids in joints or sewers and other underground structures. Hydrophilic gel grouts will not stick to concrete and are not recommended for moving cracks. They are used for sealing sewer joints and manholes, and other underground applications. Due to their relatively short gel times and high viscosities compared to the acrylics, they are usually not used in sealing lateral sewers with remote lateral packers.

4.3.2 Hydrophobic Grout Systems

Hydrophobic resins are water activated systems that require roughly 4% water to start the chemical reaction. They have expansive qualities, ranging from 6 to 20 times expansion and are generally referred to as “foams”, sometimes as rigid foams. Due to the low water content they are considered non-shrink, as the foam matrix has so little water that even in extremely arid conditions they will maintain their cured form. One of the other characteristics is that they are controllable. Unlike hydrophilic, they have an additive that is referred to as an accelerator, as it allows the applicator to control the IR cure time from 1 to up to 10 min. The accelerator is not to be confused as a catalyst as it does not start the reaction, but allows it to be controlled. Before the reaction can begin, the accelerated resin must still come into contact with water to start the reaction.

Two-component systems can have high expansive properties with many of them capable of curing to a foam density of 96 kg/m³. Unlike the hydrophobic or hydrophilic systems, they do not require water as a catalyst as the reaction is started when Part A comes into contact with Part B in a static mixing tube. They are generally much faster reacting systems and can reach up to 25 times expansion in as little as 7 to 10 seconds. With the high expansion rates and extremely fast reaction times, they can have the potential to move structures and require extreme care when using.

4.3.2.1 Applications

Typical applications include sealing cracks/joints, creating a water impenetrable barrier between the backside of a structure and the soil matrix from the negative side. A major advantage to sealing active leaking cracks/joints is that material is water activated as opposed to most materials that require the water intrusion to be eliminated before the repairs can be done. The cured resin is designed to accept movement, allowing the materials to be successful in applications subject to movement due to seismic activity, contraction/expansion or movement designed into the structure where a rigid material like epoxy is prone to failure. Many below-grade structures start out with a membrane installed on the positive side as waterproofing. While these systems have proven to be effective, they, like many others, have a lifespan anywhere from 15 to 30 years. Once the systems lifespan is exceeded the owners are faced with the costly replacement that includes excavating to expose the failed system, removing and replacing. With the polyurethane systems a series of holes are drilled through the structure from the negative side and the resin is injected to create a monolithic barrier between the backside of the substrate and the soil. This application provides a long-term repair at a considerable savings.
4.3.2.2 Limitations

As with all materials, Polyurethanes also have limitations. Hydrophobic polymers usually have better chemical resistance. To insure proper cross-linking during the reaction, water should be tested to ensure a pH level of 10 or less. A pH close to neutral (7) produces the most ideally cured polymers. A pH below 7 slows down the reactivity and too far below 7.0 will “kill” the reaction. Higher pH will increase reactivity up to a pH 8-9, but after that will begin to degrade the quality (the water holding ability) of the cured polymer as the pH increases. Recall that pH 7 is neutral and as the pH falls exponentially toward 1, it becomes a stronger acid. As the pH climbs above 7, the same is true for increasing alkalinity up to the maximum of 14. While a water temperature of 10°C or higher is preferred, the materials have been successfully used with water temperatures near freezing. Below 10°C the material will steadily decrease its cure rate as well as its physical characteristics, and once the water begins to crystallize, the resin cannot absorb it and the reaction will not occur.

Hydrostatic pressure has similar effects on the resins. Starting at one atmosphere, the material reaction time as well as the expansion and swelling begins to lessen, and after 10 atmospheres they will still react, but at an extremely slower rate and without any expansion or swelling. The water/diisocyanate reaction creates carbon dioxide and hydrostatic pressure controls the amount of CO₂ that can dissolve into the water column. High pressure and colder water temperature will produce the least amount of foaming in the cured polymer while lower pressure and warmer water increase the foam yield. Grouts that reacted on a “desktop” at room temperature without any containment form the maximum amount of CO₂ hence the larger amount of cured foam. High concentrations of hydrocarbons will not allow proper cross-linking of the molecules and the material will not react. Hydrophobic foams tend to be rigid and some will not stretch, meaning they are not the best product for a moving crack. All urethanes are adversely affected by UV rays and high temperatures, say in excess of 40°C.

5.0 Tools and tackles used for pressure grouting

Following are tools and tackle used for pressure grouting:

- Air compressor with a capacity of 3 to 4 m³/ min and with a pressure of 3 to 5 kg/cm².
- Grout injecting machine or grouting pump with inlet and outlet valves (Fig. 2 & 3). It should be capable of injecting cement grout up to 5 kg/cm².
- Hydraulic pressure gauges (Fig. 4) showing pressure commonly in bar (1 bar = 14.5 psi or 0.98 kg/cm²)
- An air tight, pressure mixer chamber, with stirrer for proper mixing of the grout and keeping it in proper colloidal suspension during grouting.
- Flexible pressure hose pipes for transmitting grout from pressure chamber to ports embedded in the masonry/concrete (Fig. 2 & 3).
- Drilling equipment, pneumatic or electric (Fig. 5), for drilling of holes up to 25 mm dia.

![Fig. 2: Injection packer for high pressure grout](image)

![Fig. 3: High pressure injection pump](image)

![Fig. 4: Hydraulic pressure gauge](image)

![Fig. 5: Electric drill - Heavy duty - power output - 1050 watts and drill speed - 350-600 rpm with variable speed control](image)
5.1 Pressure grouting procedure for filling cracks/voids/cavities/leakages

- Cementitious grouts usually require larger holes and nipples for injection than do resinous grouts.
- Drill a hole of 4 – 5 mm larger than the dia of injection packer/nozzle and insert it up to a depth of 80 mm. If cracks are clearly visible, injection ports can be installed at appropriate interval by drilling directly into the crack surface.
- Polymer modified mortar/epoxy putty shall be used to fix and seal the sides of the packer/nozzle (Fig. 8).

- PVC grouting nozzles of 130 mm length & 10mm outer dia, with a stopper at the outer end, shall be fixed on the concrete surface @ 500 mm to 1m C/C or at the particular location of crack/void/water leakage area.
- If the cracks are more wide between port to port then seal the crack with epoxy putty. Then the surface of the crack between ports is allowed to cure.
- After 24 hrs of fixing of nozzles/packers, grouting operation shall be carried out.
- The cementitious grout shall be prepared with mix of cementitious material & non shrink grouting chemical. W/C ratio shall be maintained not more than 0.45.
- For epoxy and polyurethane injection, Part A which consists of a resin and Part B which is a curing agent or hardener need to be mixed together.
- Both pumping rate and pressure should be monitored during injection. It depends on type of grout materials, crack and void network inside the concrete.
- Grout mix shall be prepared of good consistency by mixing thoroughly with a paddle mixer (Fig. 9) for ease in passing through grouting pipe.
- For two component materials of epoxy or polyurethane, both the components to be mixed as shown in Fig. 10.
- The grouting shall be done under pressure of 3 to 5 kg/cm² using grouting pump in normal condition and a pressure of 8 to 10 kg/cm² in some specific conditions (Fig. 11 & 12).
- Injection begins with lowest elevation port and proceeds along.
• Grout shall be pumped till the time it flows into the structure, filling all the gaps/voids/cracks inside. The grouting activity should be stopped once it becomes very difficult to further pump in or at a time when the grout material oozes out from adjacent grout point through the installed perforated nozzles/packers. The consumption of grout in each hole shall be recorded.

• Once completed and grout is completely set, the projected parts of the nozzles shall be removed, surface shall be cleaned and finished smooth. If PVC nozzle is being used the same should be cut and sealed with epoxy putty.

• Wherever packers are being used the same can be removed, cleaned and reused.

• Seal the grout holes with epoxy putty or polymer modified mortar.

5.2 Pressure grouting for repair of raising dampness

• Pressure injection of non-aqueous silicone injection grouts of water-repellent solutions into tubes sealed into holes in the masonry.

• Drill the holes with a 12 - 15 mm drill bit at 45° angle, penetrating half width of the wall (Fig. 13).

• Start from skirting level with 300 mm distance between each hole horizontally and 300–400 mm vertically from ground level, at c/c.

• Fix the perforated PVC nozzles and seal them secure with instant leak plug material and allow it to set for one day before starting next work.

• Use grouting pump with minimum 2 bar (2 kg/cm²) pressure capacity to pump the silicate based grout (Fig. 14).

Fig. 12: Grouting in action with a hand pump in a deck slab of highway culvert with epoxy injection grouts

Fig. 13: Drilling the hole by a drill bit at 45° downward

Fig. 14: Injection with silicate based injection grouts for raising dampness in masonry wall

5.3 Grouting bed plates

Grouting is often performed under bed plates and machinery bases to ensure uniform and firm support, in many instances, these bases must be set and maintained at precision tolerances. Accordingly, the grout material must be strong and durable, as well as stable, free of bleed, and essentially non-shrinking. Both cementitious and resinous materials are used. The resinous grout should not be used where thickness is more than 13 mm because of its limiting properties. Typically base plate grouts do not require much pressure to be simply forced from one side to the other; however, the quality of the work will be improved if positive pressure can be used and maintained for a short period of time. This will typically be within a range of 0.7 to 1.7 bars.

Generally the grouting under base plates are done in following cases:

• Anchor bolt base plate grouting (Fig. 15)

• Column base plate grouting (Fig. 16)
The schematic diagram of grouting below base plate and foundation bolt pocket is given in Fig. 17. The step by step application procedure given as follows:

![Schematic diagram of grouting below base plate and foundation bolt pocket](image)

**Fig. 17:** Schematic diagram of grouting below base plate and foundation bolt pocket

### 5.3.1 Primary Grouting
- Bolt Pocket Grouting may be done, if possible, below the top level of the concrete foundation.
- Provide a “key” for secondary grouting.

### 5.3.2 Surface preparation
- Remove the dust, unsound material with air compressor
- Wet the surface with water for at least 24 hours prior to the grouting
- Remove the excess water

### 5.3.3 Secondary grouting
- Align base plate in line and level.
- Shuttering a must – with a hopper for pouring the grout.
- If length is too long divide into sections.
- Sections can be made of wooden shutters or separations made with mortar containing general purpose grout.
- Always ensure surface saturated dry (SSD) condition.
- After one part hardens, grout the adjacent section in, say, maximum gap of 8 hours.
- Pouring by gravity; increasing height of tremie / hose.

### 5.3.4 Check during grouting application
- Mix the grout with given water to powder ratio. Add 75% of the water first, mix it for three minutes.
- Add balance water and mix it further for two minutes till it becomes a homogeneous mix.
- Pour the grout from one end only.
- Do not stop until complete grouting is finished.
- Application temperature should be between 5 - 40°C.
- Cure the grout for minimum seven days.

### 5.4 Anchor Fixner Grouting
Anchor fix grout is a two-component polyester resin & hardener-based anchor fixing grout for anchoring of bolts from 8 to 50 mm dia. into concrete.

#### 5.4.1 Viscosity and thixotropy
For vertical holes with the opening upward, a liquid adhesive can be used. A liquid adhesive requires less time to place than a paste or thixotropic or psuedoplastic adhesive and it is much less likely to trap air in the bond line. For vertical overhead and horizontal holes, a thixotropic or psuedoplastic paste adhesive is more suitable because it will not require containment to keep it from running out of the hole. However, it must be capable of being pumped from the bottom (back) of the hole toward the front of the hole to avoid trapping air bubbles in the bond line. Air bubbles would reduce contact area and result in a weakened bond.

#### 5.4.2 Application guidelines
- Hole diameters for anchor fixing normally used are 3 to 12 mm greater than the bolt, dowel, or insert diameter. In all cases, the smaller the annulus between the insert diameter and the hole diameter, the lower the possibility of creep failure. As the annulus dimension increases, the potential for creep failure under constant load increases.
- To develop the full strength of a steel anchor or a reinforcing bar, as opposed to inducing a failure in the concrete, the steel should generally be embedded to a minimum depth of ten times its diameter.
- Anchor spacing should allow a sufficient quantity of anchors to transfer the desired loads from the attached members without development of excessive stress interaction through the concrete between the anchors.

#### 5.4.3 Important strength considerations
- Pullout strength- Pullout strength is generally determined by applying an axial tensile load to the anchorage until tensile failure occurs. The ability of the concrete-anchor system to develop full pullout strength of the anchor as determined by ASTM E 488 depends mostly on the bond strength of the adhesive and the cleanliness of the hole. This test evaluates the ability of the adhesive to bond and cure under the conditions of moisture and surface preparation actually encountered in application.
- Creep resistance- Many inserts that are bonded into concrete are put under a constant load. Examples are fixtures being hung from anchorages and torqued anchor bolts. Therefore, creep resistance should be carefully considered. For critical applications pre-testing of a mockup is recommended because no standard test methods are currently available.

### 5.5 Contact grouting
It involves the filling of generally small voids behind or under a rigid lining or conduit. The grout is injected through holes that penetrate the lining. Voids typically filled by contact grouting are relatively thin and cementitious suspension or slurries. Generally, the intent of contact grouting is to increase the structural integrity of the structure. Examples of where contact grouting may be used include the following:
- Within pressure tunnels to prevent expansion of the tunnel liner under pressure.
• Within sewer tunnels to prevent sulphate attack of concrete liners from behind the liner

5.6 Post tension tendon grouting
Prior to the stressing of post tensioned tendons, they must not be bonded to the concrete. This is typically prevented by encapsulation within a snugly fitting encasement or by installing the tendons into ducts consisting of pipe or similar tube. To bond these tendons to the surrounding structure and to protect them from corrosion, the ducts are normally grouted following the stressing operation (Fig. 18).

Fig. 18: View of post tension tendon duct for grouting

6.0 Grouting in Masonry
Voids in masonry are far more common. They tend to be more continuous, and represent a larger amount of a section’s volume than those in concrete. Injection of significant quantities of grout is thus common in these structures, so the further weight added often becomes critical and must be carefully evaluated. The most important factor for grouting in masonry structures is improving the fixity of the individual units in order to reduce the risk of their becoming loose and falling during earthquakes or other dynamic loading events.

The older masonry structures are bonded with lime mortar, which become loosened over a period of time due to ageing, environmental distresses, etc. In such cases, both cementitious and resin-based grouts are used to a large extent. While doing injection grouting, cementitious grouts are more suitable since they bond very well to older masonry structures. The injection pressure should be very low from 0.3 to 0.7 bars in old masonry structures to avoid any displacement of elements that may occur with high pressure. For larger voids and cavities, resinous foam injection grout would be more suitable. For leaking at joints in masonry structures, it is wise to do repointing or resealing of the joints with cementitious grouts. Tile grouting should be done with polymer-modified cementitious or epoxy grouts.

7.0 Quality Control and Quality Assurance
While doing grouting, different colours should be marked for primary as well as secondary grouting points. Each hole should be numbered for identification. The consumption of material during injection varies at each grouted location, which depends upon the size and nature of voids and cracks inside the concrete structures. After injection is completed, the consumption of material has to be noted and signed by the contractor and engineer-in-charge. All the records and forms related to grouting should be made available at each location, such as grout pressure, volume injected, and the time at regular intervals. Other data should be noted, such as date, time, hole number, etc. The detailed method statement should be given to the contractor, showing what to do and how to do it. The detailed technical specification should include layout of grout points, spacing/location, number of holes, angle of holes to be drilled, diameter and depth of holes that are to be accepted, grout mixer requirements, material requirements satisfying desired properties as per the standards, pumping rates, grout pressure required and method of pressure selection for individual holes, requirement for monitoring and recording the injection parameters during grouting, and requirement of skilled personnel. The performance specifications after the grouting need to be mentioned clearly. The effectiveness of grouting must be monitored by some non-destructive test such as ultrasonic pulse velocity or by taking a core sample at the grouted location and checking the compressive strength of the core sample before and after grouting at that particular location. This will help in monitoring the improvement of the quality of concrete after the grouting.

Wherever there is leakage, the same technique can be used to check if leakage has stopped after the grouting is completed. There may be some locations where leakage might occur even after grouting is done and this can be stopped after the secondary grout at the leakage spots. The extreme environment poses challenges to grouting activities. Virtually all grout mixtures will react, set, or cure more rapidly as the temperature increases. Beyond a certain temperature, an immediate or flash set will occur. Conversely, reactions are much slower as the temperatures decreases. In such extreme environments, chemical solutions of resinous categories should be added as modifying agents to control the reactions. The specification is the most important part of any job, which describes the method of working, quality parameters, acceptance limit, method of measurement, method of payment, etc. In the absence of detailed general and technical specifications of the job, disputes can arise between the owner and the contractor.

8.0 Conclusion
The grouting offers many services such as control of water leakages, strengthening of structural and non structural elements along with a wide range of structural applications for both concrete and masonry structures. Understanding the properties of different grout materials, specifications, formulation, step by step application methods, required pressures, types of pumps & mixers, limitations of each type of grouting methods will help to increase the durability of each grouting system.
Cementitious Grouting under base plates and fixing of rails in an Automatic Storage Yards - A case study

[Excerpts from Pidilite case studies]

1.0 Background
M/s KONE Elevators has got a factory unit at Ayanambakkam, near Chennai. They are among the top three leading manufacturers of elevators worldwide. They planned to install Automatic Storage yards with steel columns foundation and rail base plates in consultation with “CRN”, a leading consultant based at Chennai. Grouting was to be carried out in two rails of the sizes 21.55 m long, 300 mm wide and 40 mm depth and 5.5 m long, 350 mm wide and 60 mm depth, for the 1st and 2nd rail respectively. Grouting was also to be carried out for the base plates of 28 steel columns as shown in Fig. 1.

Fig. 1: View of rail 1 and base plates of steel columns

2.0 Grout Material
The cementitious grout selected was PAGEL VI/50 which consisted of basalt sand and gravel up to 8 mm. The particle grain size of the grout material was up to 5 mm. The water to powder ratio was 0.12. The compressive strength of the grout was 38 MPa & 75 MPa respectively, after 24 hours and 28 days. The work was carried out by M/s Sri Jaivarshini Chemtec, Chennai. Properties of this grout were as follows:

- Heat-resistant up to 400°C
- Capable of high flowability, it can be used as grouting mortar or, depending on the quantity of water, as tamping mortar
- Ready to use, needed water only to be mixed with
- Free of chlorides
- Does not shrink, develops a controlled increase in volume with force locking bond between concrete foundation and machine plate
- Resistant to freeze thaw-cycles, impervious to water and resistant to oil and chemicals

The general properties of the grout which require specification and control are given in Table 1.

### Table 1. Test methods & requirement for grout properties

<table>
<thead>
<tr>
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<th></th>
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</thead>
<tbody>
<tr>
<td>Flowability</td>
<td>Cone Method</td>
<td>≤ 25 s</td>
<td>≤ 25 s</td>
</tr>
<tr>
<td></td>
<td>Immersion test</td>
<td>≥ 30 s</td>
<td>-</td>
</tr>
<tr>
<td></td>
<td>Grout slump</td>
<td>-</td>
<td>≥ 140 mm</td>
</tr>
<tr>
<td>Bleeding</td>
<td>Wick-induced inclined tube</td>
<td>-</td>
<td>≤ 0,3% initial vol.</td>
</tr>
<tr>
<td>Sedimentation</td>
<td>Glass cylinder</td>
<td>≤ 2% volume</td>
<td>-</td>
</tr>
<tr>
<td>Setting time</td>
<td>Grout density</td>
<td>-</td>
<td>&lt; 5%</td>
</tr>
<tr>
<td>Volume change</td>
<td>Wick-induced test</td>
<td>-</td>
<td>&gt; - 0,5% &lt; + 5</td>
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<tr>
<td>Compressive</td>
<td>EN 445:1996</td>
<td>28 days ≥30 MPa</td>
<td>-</td>
</tr>
<tr>
<td>strength</td>
<td>EN 12190</td>
<td>-</td>
<td>7 days ≥ 27 MPa</td>
</tr>
</tbody>
</table>

3.0 Application

3.1 Form work
The form work must be of rigid construction, made of wood or steel, with sand or dry mortar placed around the concrete base carefully to prevent leakage. In this case, wooden form work was prepared and fixed in position as shown in Fig. 2.

Fig. 2: View of form work and the rail being placed firmly before grouting

3.2 Surface Preparation
The surface was cleaned thoroughly; all loose and unsound materials were removed by an air compressor (Fig. 3). Prior to grouting, the surface was wetted (Fig. 4) continuously with water for approximately 6 hours till saturation.
3.3 Mixing
The grout selected was a ready-to-use grout, which required only water to be added. For better flow and workability, the temperature of mixing water should be above 0°C and below 25°C, which was maintained in this case. The required quantity of water as per water to powder ratio of 0.12 was measured and two thirds of this was filled into a container and added with the dry mortar and mixed for about 3 minutes. Then the remaining water was filled in and mixed for another 2 minutes. Since the quantity was small, the mechanical mixing was done in a small container (Fig. 5). In case of a large quantity of grout material, a concrete mixer would have been taken for mixing. Then the grouting took place immediately after the mixing.

3.4 Grouting
The mixed grout was placed from one side corner only and was poured continuously. When grouting is taken in large areas, the pouring should be done from the middle using a pipe or a funnel. For installation of machine, first the grout was filled into anchor bolt pockets up to the top of the anchor bolt pockets and then the underside of the base plate of the machine. Fig. 6 shows pouring of grout to fix the rail.

4.0 Pidilite Grout Materials
Pidilite has wide range of grout products from Pagel for different specific applications and environment conditions which are given as follows:

4.1 Pagel Free flow Grouts
- Pagel V1/10 - Premium grout for critical applications (5–20 mm grouting height)
- Pagel V1/50 - Premium grout for critical applications (20–120 mm grouting height)
- Pagel V12HT - Premium high early strength (16 hours ~ 30 MPa) grout for critical applications
- Pagel V1/160 - Premium grout for critical applications (> 100 mm grouting height)
- Pagel V1 - Universal grout for precision machines of any kind
- Pagel V12 - High early, high strength grout for all foundations

4.2 Pagel Speciality Cementitious Grouts
- Pagel V1A/40 - Steel fibre reinforced grout for heavy load foundations
- Pagel V15/50 - Heat resistant grout
- Pagel V1A15/50 - Steel fibre with basalt grout
- Pagel V2/40 - Quick setting grout
- Pagel V14/40 UW - Under water grout

4.3 Dr. Fixit Grouts
- Dr. Fixit Pidigrout 5M - Medium strength, non-shrink, cementitious free flow grout for machine foundations
- Dr. Fixit Pidigrout 10M - High strength, non-shrink, free flow cementitious grout for foundations up to 70 mm
- Dr. Fixit Pidigrout EG-3 - High early strength and chemical resistance high flow epoxy grout for heavy dynamic and mobile loads

5.0 Conclusion
As it can be seen that there is a wide range of grout products available in the market, but the selection of grout materials should be such that it satisfies all the required properties, meets the requirement of standard specifications, and is followed by a step-by-step procedure of application. The grouting work of foundation for steel columns and rail base plates of Automatic Storage yards was completed successfully.

(Refer our ReBuild publications Vol.3, No.1, 2009 and Vol.4, No.4, 2010, pp.7-10 on grouting for further studies)
Grout formulation and Specifications


1.0 European Standard and Guidelines for Grouts for Pre-stressed Structures

The currently applicable European Standard EN 447 “Grout for prestressing tendons – basic requirements” provides general guidance and define test procedures for quality control of grouts, but there is no guidance for grout formulation. Therefore within the research project “Improvement of properties of grouts for prestressing tendons and/or ground anchors” in the European COST Material Action 534 methods for testing rheology, bleeding, setting, expansion and mechanical strength of the grout were performed and evaluated. Research was carried out on the influence of different admixtures such as a superplasticizer (SP), an expanding agent (EXP), corrosion inhibitor (INH), etc., on physical and mechanical grout properties. The overall goal was to design an optimum grout which combines desirable fresh and hardened properties with good corrosion protection and to develop rules for grout formulation. Some of the most interesting results and conclusions are presented in this paper.

Portland cement grout is used in post-tensioned and retaining structures to provide bond between the tendon/anchor and the surrounding concrete and also to fill the voids between protective duct and prestressing strand which suppresses the flow of water and chloride ions. Grout is pumped into the space between the steel and the surrounding duct, where it hardens to transmit the stresses from the steel to the concrete. Therefore, the properties which are of interest and require specification and/or control are: rheology and flow; dimensional and material stability; setting and strength development; and durability.

1.2 Required Properties of Grout

1.2.1 Rheological Parameter

Rheological parameters of the grout should be tested and controlled to ensure that grout will fill the protective duct before it is sealed. The most common method for testing grout fluidity is the cone test. One point of criticism of the prescribed test method is that it is a single-point method. This method cannot be of the grout during 3 hours, within which grout needs to be injected into the duct. That is why during the project rheology tests were performed using a scientific instrument (coaxial rheometer), with which it is possible to monitor the change of shear stress of grout in time.

1.2.2 Bleeding and Volume Change

Bleeding and volume change of the grout should be tested to ensure that no voids are formed after the duct is filled with grout. The results obtained with methods for bleeding and volume change prescribed in EN 445:1996 do not represent the true conditions inside the duct because no strands are present, which have a strong influence. The fib-guideline prescribes the inclined-tube test (for bleeding) or the Wick-induced test (bleeding and volume change), which were taken over in the EN 447:2007. These methods are more representative, but are, on the other side, rather difficult to carry out in normal laboratories and even on site.

1.2.3 Mechanical Properties Testing

Mechanical properties of grout should be tested to ensure mechanical performance and stress transfer between prestressing strand and concrete. During research on the project it was concluded that compressive strength was usually satisfied regardless admixtures that were added into grout mixture.

1.2.4 Corrosion Testing of Grout

Corrosion behaviour of grout/prestressing strand system should be tested to verify that grout will resist aggressive substances and protect prestressing steel from corrosion. In order to evaluate corrosion protection capability of different grout mixtures samples with embedded prestressing steel were prepared and potentiostatic anodic polarization was performed. From corrosion testing it was concluded that grout with low chloride diffusion coefficient, good homogeneity and volume persistency could assure longer durability of prestressed structures.

2.0 Post-Tensioning Grout Bleed, Duct and Anchorage Protection Test

Due to substantial problems with product quality of grout materials, the Department of Transportation, USA has revised all specifications concerning post-tensioning corrosion protection. New products for grouting post-tensioning ducts have recently become available. This article focuses upon three parts of the post-tensioning system: cementitious grout; internal duct; pour-back material.

2.1 Grout Testing

The current specification for post-tensioning grout does not differentiate between horizontal and vertical grout applications. In order to broaden the specification to address both applications, a relationship needs to be developed to associate laboratory test (Schupack) and field simulated test (wick induced bleed). The laboratory test can then be used with information collected from the testing to quantify the difference between horizontal and vertical applications. If a correlation is established between the vertical or inclined tests and the pressure tests, then the pressure tests can be used to test the
bleed properties of new materials. In addition, the technique used for the simulated field test needs to be verified. The single wick, triple wick and inclined wick will be tested to determine the most severe condition for grout bleed.

2.2 Corrugated Duct Test
Corrugated duct is currently available with three distinct styles of ribs. The first type of duct has ribs that are parallel and oriented perpendicular to the axis of the duct. The second type has spiral ribs. The third type has parallel ribs similar to the first, but with four additional longitudinal ribs that are parallel to the axis of the duct and equally spaced radially around the circumference. This test can examine the effect of the corrugations on the bleed properties of the grout. Three 15 m long ducts should be grouted with the same prepackaged grout. After acquiring the required strength, the duct should be cut into segments and be examined to determine if the corrugations would have an adverse effect on the completeness of grouting.

2.3 Epoxy Pour-Back Test
A full scale mock-up of a combined multiple anchorage pour-back should be constructed and subjected to temperature variations. The test would determine if shrinkage and differential volume change between the materials would cause cracking of the pour-back.

3.0 Masonry Grout
ASTM C476 has provisions for establishing grout proportions on the basis of specified compressive strength. The specified compressive strength must be at least 15 MPa. Grout mixers meeting the proportion table of ASTM C 476 have high cement contents and tend to produce much higher strength than specified compressive strength requirements of ASTM C 476, ACI 530 or model codes. Two types of masonry grouts are defined in ASTM C 476: fine grout with aggregates smaller than 10 mm and coarse aggregate sizes up to 12 mm. Choice of grout types depends primarily on the clear dimensions of the spacing being filled by the grout.

The consistency of grout affects its strength and other properties. It is critical that grout consistency permit the complete filling of void space without segregation of ingredients. Consistency of masonry grout may be measured with a slump cone ASTM C 143 and slumps of 200-275 mm are generally required for both fine and coarse grout. Self consolidating grout is a highly fluid and stable grout mix that does not require consolidation. These grouts are tested using the slump flow test, ASTM C 167, which measures the spread of the grout using the slump cone.

For other types of grouts without aggregate, or only fine aggregate passing a No. 8 sieve, consistency, is best determined with a flow cone (ASTM C 939). For flow values exceeding 35 seconds, use the flow table in ASTM C 109, modified to use 5 drops in 3 seconds. Masonry grout (block fill) for strength test specimens should be cast in molds formed by masonry units having the same absorption characteristics and moisture content as the units used in construction (ASTM C 1019). Never use nonabsorbent cube or cylinder molds for this purpose.

Strength of other types of grout is determined using 50 mm cubes as per ASTM C 942. This standard allows for field preparation, recognizes fluid consistency, and also affords a means for determining compressive strength of grouts that contain expansive agents or grout fluidifiers. This is extremely important since expansive grouts can lose substantial compressive strengths if cubes are not confined. However, cylindrical specimens 150 mm x 300 mm or 100 mm x 200 mm may give more reliable results for grouts containing coarse aggregate. Special application grouts often require modification of standard test procedures.

4.0 Assessing the Stability of Grout
The injection and filling of post-tensioning ducts have been carried out since last 5 to 6 decades. It was seen that post-tensioning tendons were often covered by a film of grout, indicating that grout had been present during construction but had fallen back. The bleeding was the major cause for failure of the grouts in such cases.

The specification of grout stability test is to measure the bleeding of the grout mix. This test also enables to measure the volume change of the grout. The grout need to be of high-performance having zero bleed and some limited expansion as per the provision of the standard which may be practically very difficult to achieve at site. Hence, it may be feasible to set a level closer to 1 % bleed as acceptable limit. With regard to expansion, it would be prudent to set a lower limit of 1% to compensate for early age shrinkage. In addition, it should be set an upper limit not higher than 5%, preferably lower, as excessive expansion can be accompanied by internal cracking and formations of cavities in a closed container.

5.0 Conclusion
Testing of physical and mechanical properties of grout is necessary to determine suitable grout, which will ensure durable prestressed structures and other applications. Laboratory and on-site testing should be performed as part of quality assurance and quality control of materials built into the structures. However, some of the tests methods that are prescribed in the standards and guidelines may not be suitable for laboratory and do not always fully deliver the necessary information. This need to be tested at site with some modifications that should be suitable for the site or some proto-type tests under the close supervision of experience engineers and executed by skilled operators who understand the correct procedures.
The Institute’s Activities

Train-The-Trainer Programme on Waterproofing Applications

**Topic:** Waterproofing Applications  
**Date:** 17 - 21 November 2015  
**Participants:** Engineers and supervisors from Nina Waterproofing, Mumbai and Percept Waterproofing, Bangalore  
**Venue:** Pidilite Taloja plant, Navi Mumbai  
**Coordinated by:** Dr. Fixit Institute & CC Division of Pidilite

Corporate Training Programme

**Topic:** Waterproofing, Building Maintenance and Repairs  
**Date:** November 2015  
**Venue:** Rustomjee Academy for Global Careers (RAGC), Mumbai  
**Topic:** Structural Assessment, Repair & Maintenance of Civil Structures in Marine Environment  
**Date:** 9th December 2015  
**Venue:** Estate office of Refinery Petrochemical Complex of Indian Oil Corporation Limited at Paradeep, Odisha

Corporate Training Programme

In addition to the above scheduled programmes, we do organize separate corporate training programmes on specific topics as per the needs of the customer

Mr. John and Mr. Bill from NRCA demonstrating waterproofing application systems  
Training Session at IOCL, Paradeep, Odisha

Dr. Fixit Experience Centre

A first-of-its-kind knowledge sharing & research centre in the field of waterproofing for the construction fraternity was established on 22nd September 2015 by Pidilite Industries Limited at 308 Llyods Road, Pudupet, Gopalapuram, Chennai 600 086.

View of Dr. Fixit Experience Centre  
Prof. Dr. Ravindra Gettu of IIT Madras inaugurating the Experience centre with Shri Madhukar B. Parikh Chairman and Mr. Sanjay Bahadur Global CEO, CC Division of Pidilite Industries Ltd  
Internal view of Experience centre showing alternet waterproofing systems

Similar Experience Centre has also been established at Cochin, Malabar Willingdon Island, Kerala, Cochin-682 009

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